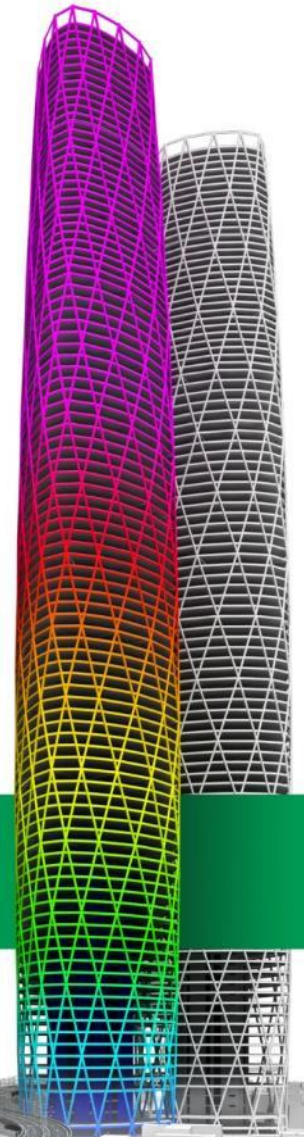




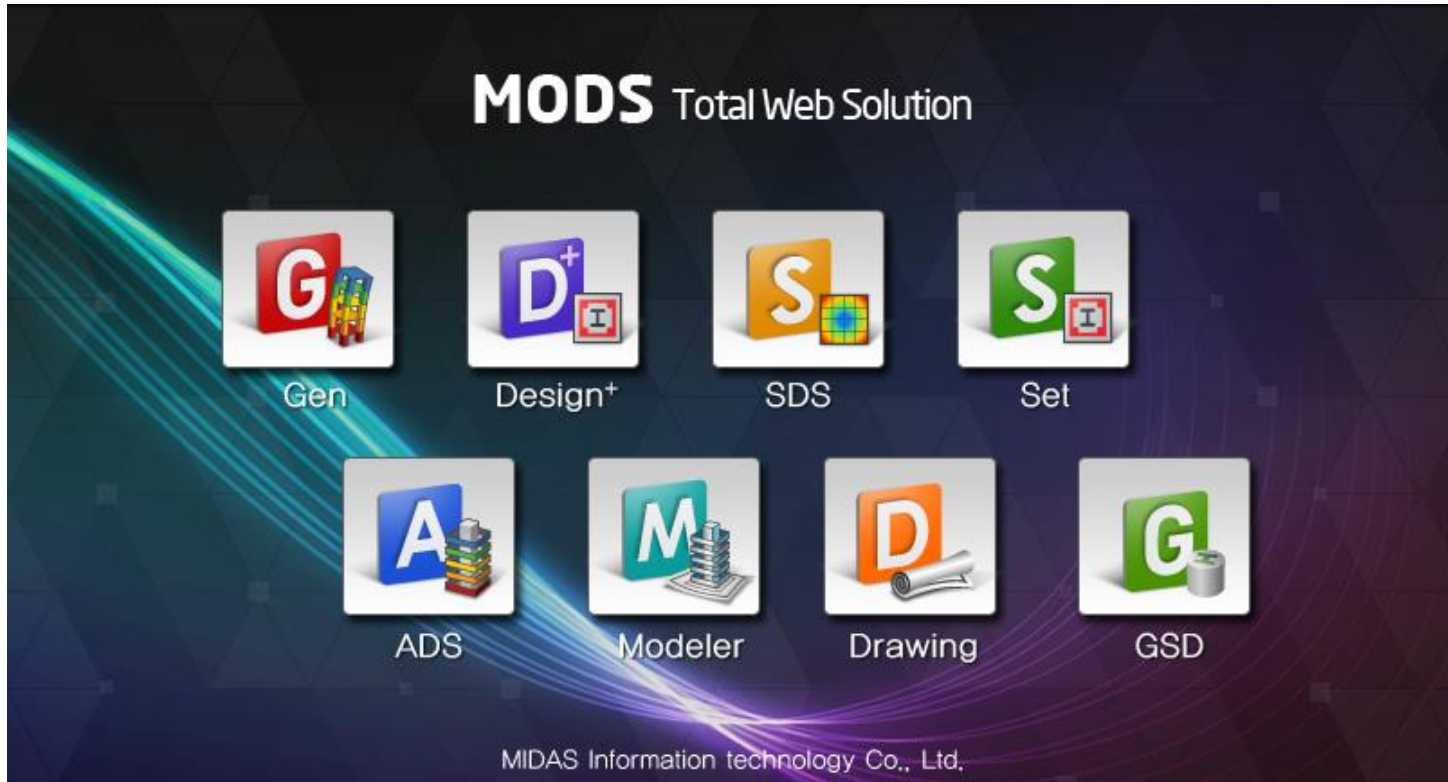
MODS 2024(Gen V945) Release Note. 2023년 11월

Integrated Design System for Building and General Structures



Product Version

MODS 2024 2023. 11. 13



기본/서비스 모듈

-  **midas Gen 2024**
(V945 R1) Version Up
-  **midas Design+**
(V495 R1) Version Up
-  **midas SDS**
(V410 R1) Version Up
-  **midas GSD**
(V325 R1) Version Up
-  **midas Set**
(V334 R1)

부가모듈

-  **midas ADS**
(V285 R1) Version Up
-  **midas Modeler**
V200 R8
-  **midas Drawing**
V300 R7

Release Note

Part I. midas Gen 2024 (V.945 R1) 주요 개정내용

- ◆ [Seismic Performance] Fiber Hinge 가 할당된 부재의 전단 결과 출력 05
- ◆ [Seismic Performance] 층간변위비 결과 그래프 출력 07
- ◆ 구속/비구속 콘크리트를 구분하여 RC Column Fiber 단면 자동 생성 지원 08
- ◆ 비선형 해석 시 Plate Based Type Wall 유효강성 고려 09
- ◆ ASCE7-16, ASCE7-22 기준 Wind Load, Wind Pressure 지원 10
- ◆ RC 부재 Checking Result Dialog Rebar Detail 출력 12

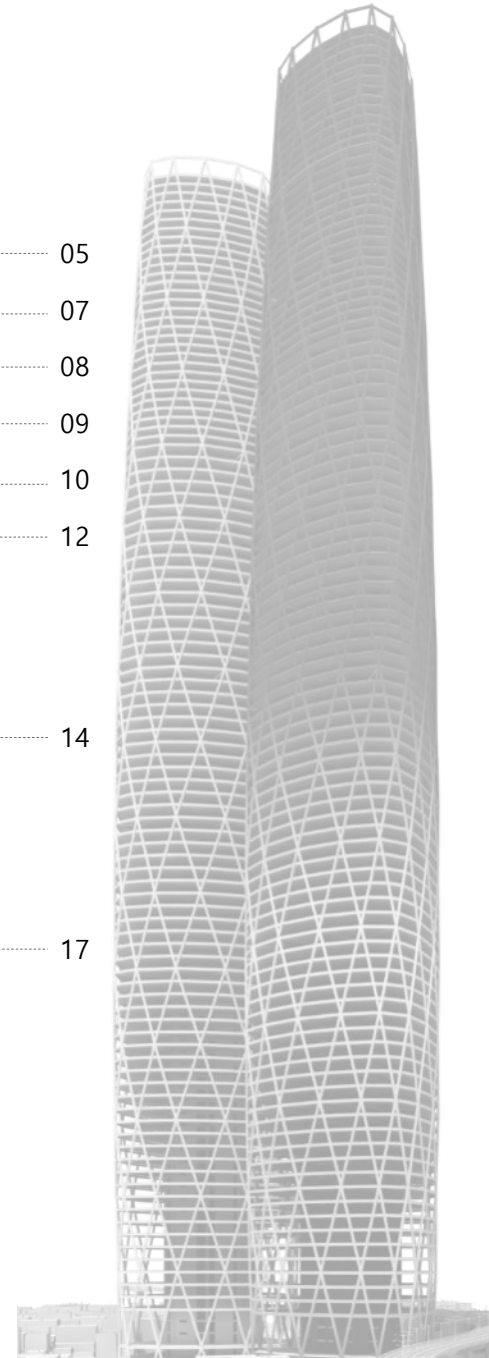
Part II. midas Design+ (V.495 R1) 주요 개정내용

- ◆ ACI318M-19, ACI318-19 설계 지원 14

Part III. midas SDS (V.410 R1) 주요 개정내용

- ◆ 콘크리트 휨강도감소계수 계산 개선 17

Part IV. 기타 개선 및 버그 수정



Gen 2024

Integrated Solution System
for Building and General Structures



Gen V.945

[midas Gen V945 R1] [Seismic Performance] Fiber Hinge 가 할당된 부재의 전단 결과 출력 - Pushover 해석

- 성능설계 지침(AIK-G-001-2021) 검토 시 지원
- Fiber Hinge가 할당된 RC Column, Wall 의 그래픽, 테이블 결과

Seismic Performance (AIK-G-001-2021) > Pushover Anal. > Performance > Shear Force Performance
Seismic Performance (AIK-G-001-2021) > Pushover Anal. > Fiber Hinge Memb. > RC Column Performance (Shear Force)
Seismic Performance (AIK-G-001-2021) > Pushover Anal. > Fiber Hinge Memb. > RC Wall Performance (Shear Force)

Pushover

Shear Force Performance

Load Cases/Combinations
POX

Step PO Step:153

Components
 Dy Dz
 Representative

Type of Display
 Deform Undeformed
 Values Legend
 Animate Mirrored
 Contour

Display
 Performance
 Member Status
 Column Wall

Performance
 IO LS
 CP Collapse

Apply Close

midas Gen POST-PROCESSOR
SHEAR FORCE PERFORMANCE

Representative

- 0.0% Collapse
- 0.0% CP
- 0.0% LS
- 100.0% IO

▪ Shear Force Performance Table Result

Story	Level (m)	Section Name	Member	Load	Step	Part	Force-Controlled Action					
							Fs	Fns	Fns+1.2(Fs-Fns)	φFn	Ratio	Performance
2F	3.6	C2	45	POX	po_0139	[133]	163.651	10.161	194.349	1911.256	0.102	IO
2F	3.6	C2	45	POX	po_0139	J[32]	163.651	10.161	194.349	1906.303	0.102	IO
2F	3.6	C2	47	POX	po_0139	[135]	-47.267	10.137	-58.747	1787.776	0.033	IO
2F	3.6	C2	47	POX	po_0139	J[34]	-47.267	10.137	-58.747	1782.823	0.033	IO
2F	3.6	C2A	46	POX	po_0139	[96]	-13.933	-13.837	-13.952	1230.020	0.011	IO
2F	3.6	C2A	46	POX	po_0139	J[26]	-13.933	-13.837	-13.952	1225.067	0.011	IO
2F	3.6	C3	44	POX	po_0139	[74]	3.968	-23.624	9.486	1261.885	0.008	IO
2F	3.6	C3	44	POX	po_0139	J[2]	3.968	-23.624	9.486	1256.932	0.008	IO
2F	3.6	C3	48	POX	po_0139	[99]	-12.683	-23.822	-10.455	1317.781	0.008	IO
2F	3.6	C3	48	POX	po_0139	J[31]	-12.683	-23.822	-10.455	1312.828	0.008	IO
1F	0	C2	2	POX	po_0139	[134]	-0.835	1.064	-1.214	1922.422	0.001	IO
1F	0	C2	2	POX	po_0139	J[133]	-0.835	1.064	-1.214	1917.469	0.001	IO

▪ Shear Force Performance Graphic Result

Story	Level (m)	Wall Mark	Wall ID	Load	Step	Axial Force Ratio	Force-Controlled Action					
							Fs	Fns	Fns+1.2(Fs-Fns)	φFn	Ratio	Performance
15F	40.8	W1	1	POX	po_0139	0.00862	-31.143	-12.888	-34.794	3269.207	0.011	IO
14F	38	W1	1	POX	po_0139	0.02027	-62.028	2.875	-75.009	3365.452	0.022	IO
13F	35.2	W1	1	POX	po_0139	0.02751	-130.552	-5.961	-155.470	3425.265	0.045	IO
12F	32.4	W1	1	POX	po_0139	0.03219	-200.994	-5.345	-240.124	3463.895	0.069	IO
11F	29.6	W1	1	POX	po_0139	0.03469	-260.622	-8.699	-311.007	3484.598	0.089	IO
10F	26.8	W1	1	POX	po_0139	0.03127	-306.659	-10.451	-365.900	3603.127	0.102	IO
9F	24	W1	1	POX	po_0139	0.03004	-346.905	-13.485	-413.589	3591.784	0.115	IO
8F	21.2	W1	1	POX	po_0139	0.02736	-380.248	-16.563	-452.986	3566.852	0.127	IO
7F	18.4	W1	1	POX	po_0139	0.02325	-403.345	-20.416	-479.931	3528.684	0.136	IO
6F	15.6	W1	1	POX	po_0139	0.01754	-417.217	-25.821	-495.496	3737.414	0.133	IO
5F	12.8	W1	1	POX	po_0139	0.00945	-407.719	-29.801	-483.303	3662.308	0.132	IO
4F	10	W1	1	POX	po_0139	0.00000	-384.884	-52.076	-451.446	3232.715	0.140	IO

[midas Gen V945 R1] [Seismic Performance] Fiber Hinge 가 할당된 부재의 전단 결과 출력 - 시간이력 해석

- 성능설계 지침(AIK-G-001-2021) 검토 시 지원
- Fiber Hinge가 할당된 RC Column, Wall 의 그래픽, 테이블 결과

Seismic Performance (AIK-G-001-2021) > Time-history Anal. > Performance > Shear Force Performance
 Seismic Performance (AIK-G-001-2021) > Time-history Anal. > Fiber Hinge Memb. > RC Column Performance (Shear Force)
 Seismic Performance (AIK-G-001-2021) > Time-history Anal. > Fiber Hinge Memb. > RC Wall Performance (Shear Force)

Time History

Shear Force Performance

Time History Load Cases Name: EQ1

Step: Max

Time Function: EQ1-1

Components: Dy, Dz, Representative

Type of Display: Deform, Undeformed, Values, Legend, Animate, Mirrored, Contour

Display: Performance, Member Status, Column, Wall

Performance: IO, LS, CP, Collapse

Apply Close

midas Gen POST-PROCESSOR

SHEAR FORCE PERFORMAN

Representative

- 3.9% Collapse
- 0.0% CP
- 0.0% LS
- 96.1% IO

▪ Shear Force Performance Table Result

Start Page MIDAS/Gen Shear Force (Time-History, Column)

Story	Level (m)	Section Name	Member	Load	Part	Force-Controlled Action					
						Fs	Fns	Fns+1.2(Fs-Fns)	φFn	Ratio	Performance
2F	3.6	C3	44	EQ1(max)	[74]	131.090	0.143	157.279	1163.290	0.135	IO
2F	3.6	C3	44	EQ1(max)	J[2]	131.090	0.143	157.279	1163.290	0.135	IO
2F	3.6	C3	44	EQ1(min)	[74]	-72.454	0.143	-86.974	1567.373	0.055	IO
2F	3.6	C3	44	EQ1(min)	J[2]	-72.454	0.143	-86.974	1567.373	0.055	IO
2F	3.6	C2	45	EQ1(max)	[133]	303.859	33.161	357.999	1751.167	0.204	IO
2F	3.6	C2	45	EQ1(max)	J[32]	303.859	33.161	357.999	1751.167	0.204	IO
2F	3.6	C2	45	EQ1(min)	[133]	-53.027	33.161	-70.264	1941.793	0.036	IO
2F	3.6	C2	45	EQ1(min)	J[32]	-53.027	33.161	-70.264	1941.793	0.036	IO
2F	3.6	C2A	46	EQ1(max)	[96]	160.860	2.199	192.593	1189.690	0.162	IO
2F	3.6	C2A	46	EQ1(max)	J[26]	160.860	2.199	192.593	1189.690	0.162	IO
2F	3.6	C2A	46	EQ1(min)	[96]	-39.076	2.199	-47.330	1489.080	0.032	IO
2F	3.6	C2A	46	EQ1(min)	J[26]	-39.076	2.199	-47.330	1489.080	0.032	IO

Fy / Fz

Start Page MIDAS/Gen Shear Force (Time-History, Wall)

Story	Level (m)	Wall Mark	Wall ID	Load	Axial Force Ratio	Force-Controlled Action					
						Fs	Fns	Fns+1.2(Fs-Fns)	φFn	Ratio	Performance
5F	12.8	HW1	12	EQ1(max)	0.00000	86.824	8.507	102.488	448.587	0.228	IO
5F	12.8	HW1	12	EQ1(min)	0.15209	-97.191	8.507	-118.330	557.903	0.212	IO
5F	12.8	HW1	13	EQ1(max)	0.00000	41.662	-0.861	50.166	336.429	0.149	IO
5F	12.8	HW1	13	EQ1(min)	0.23877	-40.574	-0.861	-48.516	415.741	0.117	IO
5F	12.8	HW2	14	EQ1(max)	0.00000	211.010	-6.375	254.487	361.146	0.705	IO
5F	12.8	HW2	14	EQ1(min)	0.41426	-138.803	-6.375	-165.288	829.999	0.199	IO
5F	12.8	HW2	15	EQ1(max)	0.00000	221.781	-6.197	267.377	455.579	0.587	IO
5F	12.8	HW2	15	EQ1(min)	0.34016	-212.740	-6.197	-254.049	829.999	0.306	IO
5F	12.8	HW3	16	EQ1(max)	0.00000	216.234	3.505	258.780	334.590	0.773	IO
5F	12.8	HW3	16	EQ1(min)	0.26983	-157.426	3.505	-189.612	655.647	0.289	IO
5F	12.8	HW3	17	EQ1(max)	0.00000	159.614	3.524	190.832	340.847	0.560	IO
5F	12.8	HW3	17	EQ1(min)	0.32383	-174.522	3.524	-210.132	629.436	0.334	IO

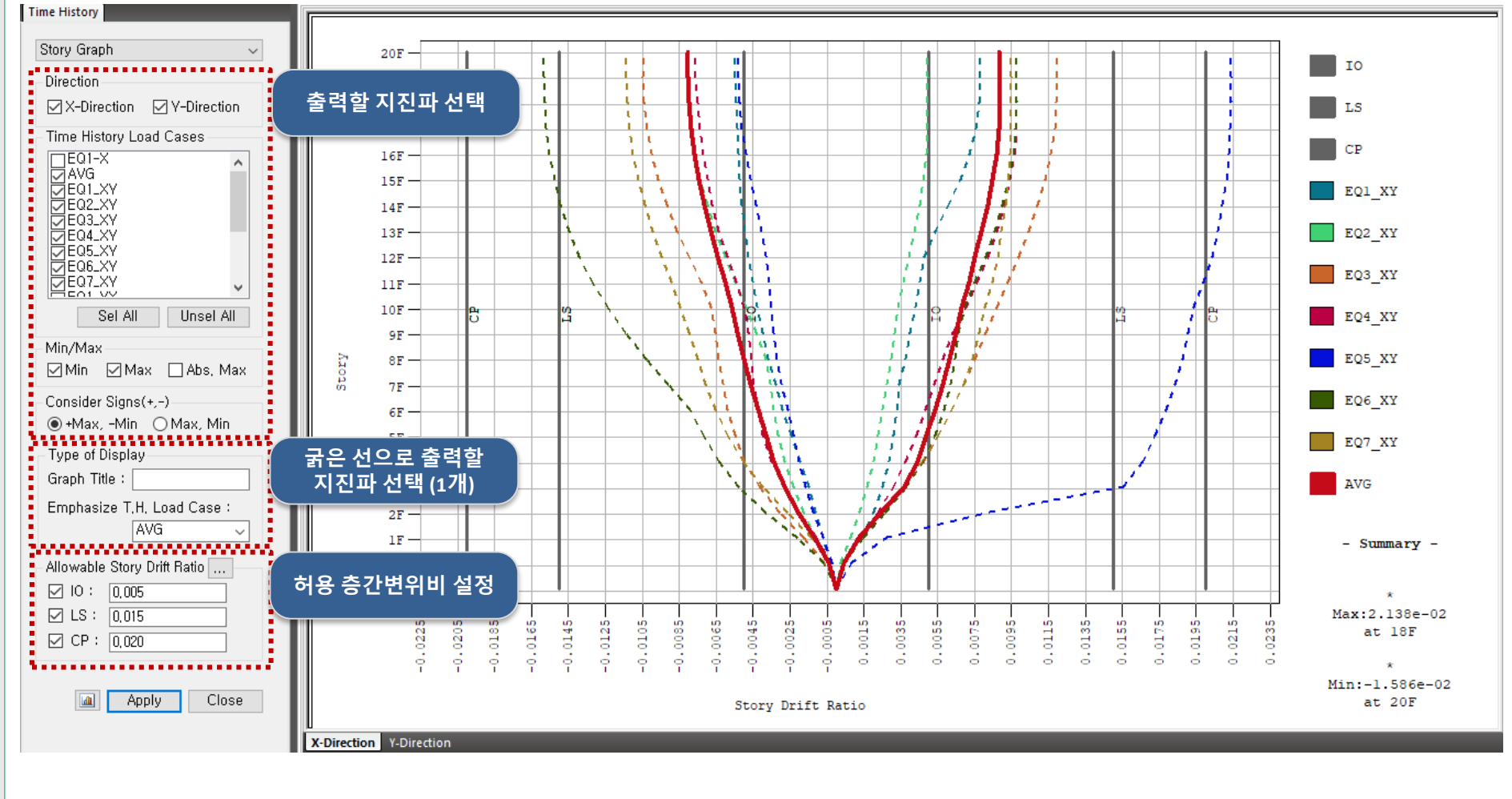
Fz

▪ Shear Force Performance Graphic Result

[midas Gen V945 R1] [Seismic Performance] 층간변위비 결과 그래프 출력

- 성능설계 지침(AIK-G-001-2021) 검토 시 지원
- 지진파별 층간변위비 및 허용 층간변위비 그래프 출력

Seismic Performance (AIK-G-001-2021) > Time-history Anal. > Story Graph



출력할 지진파 선택

굵은 선으로 출력할 지진파 선택 (1개)

허용 층간변위비 설정

[midas Gen V945 R1] 구속/비구속 콘크리트를 구분하여 RC Column Fiber 단면 자동 생성 지원

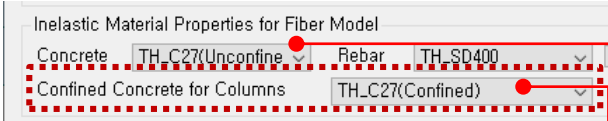
- Pushover Hinge 및 Inelastic Hinge 할당 시 자동 생성되는 Fiber Section (Column)
- Material Data에서 Confined Concrete 별도 입력하여 구분

Properties > Material Properties

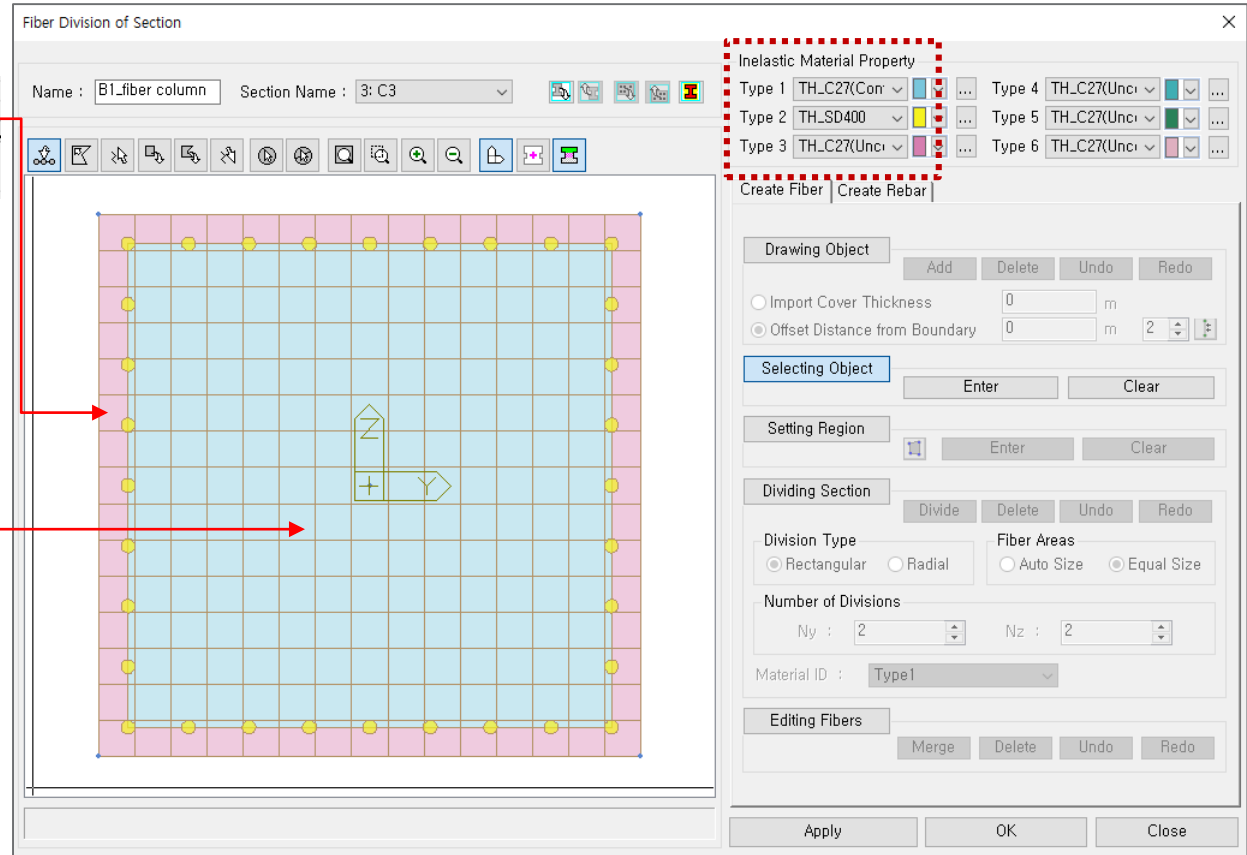
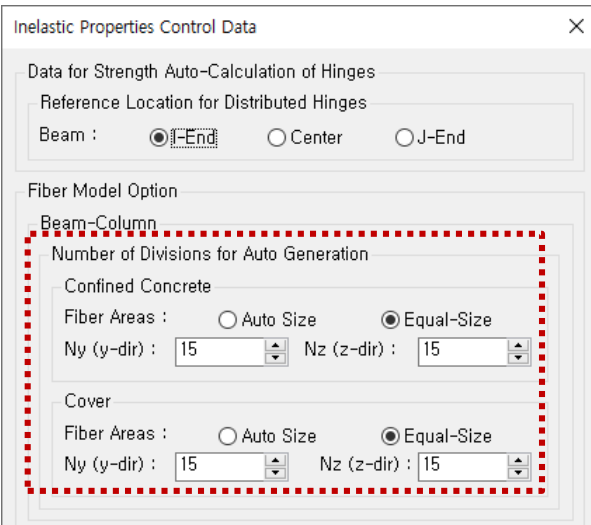
Properties > Inelastic Material > Fiber Division of Section (Beam-Column)

Pushover > Hinge Properties > Pushover Fiber Division of Section (Beam-Column)

▪ Material Data 설정



▪ Fiber 단면 자동생성 시 분할개수 설정



* Define Inelastic Hinge Properties, Define Pushover Hinge Properties 에서 Fiber Section을 'Auto Generation'으로 설정한 힌지를 할당하는 경우에 적용됩니다.

[midas Gen V945 R1] 비선형 해석 시 Plate Based Type Wall 유효강성 고려

- Pushover 해석 및 Nonlinear Time History 해석 시 Plate Based Type Wall의 유효강성 고려 여부 옵션 추가
- Plate Based Type Wall에 적용된 유효강성을 CRB Type Wall의 유효강성으로 자동 변환 지원

Load > Dynamic Loads > Global Control
Pushover > Global Control

Global Control 옵션 설정

Wall... Convert the stiffness scale factors of plate-based walls to CRB walls.

Plate Based Type Wall Stiffness Scale Factor

Wall Type
 Wall (Plate) Wall(CRB)

Inplane Stiffness Scale Factor

Shear : →

Bending & Axial : →

Out-of-plane Stiff. Scale Factor

Torsion :

Shear :

Bending :

CRB Type Wall Stiffness Scale Factor

Wall Type
 Wall (Plate) Wall(CRB)

Inplane Stiffness Scale Factor

Axial : ←

Shear : ←

Bending : ←

Out-of-plane Stiff. Scale Factor

Torsion :

Shear :

Bending :

- Pushover 해석

Plate Base Type Wall로 모델링한 경우 Pushover 해석 시, Wall(Plate) Stiffness Scale Factor를 Wall(CRB) Stiffness Scale Factor로 변환하여 적용

- 시간이력해석

Plate Base Type Wall로 모델링하고 CRB Type Wall Type Hinge를 할당한 경우 비선형시간이력해석(Nonlinear Direct Integration, Nonlinear Static) 시, Wall(Plate) Stiffness Scale Factor를 Wall(CRB) Stiffness Scale Factor로 변환하여 적용

Time History Load Cases 설정 참고

Analysis Type
 Linear Nonlinear

Analysis Method
 Modal Direct Integration Static

* 옵션 미체크 시 Plate Base Type Wall에 적용된 Wall Stiffness Scale Factor가 고려되지 않습니다.

[midas Gen V945 R1] ASCE7-16, ASCE7-22 기준 Wind Load, Wind Pressure 지원

- ASCE 7-16 기준 Ground elevation factor (Ke factor) 반영

Load > Static Loads > Wind Loads, Wind Pressure

Wind Load 설정

Add/Modify Wind Load Specification

Load Case Name : WX

Wind Load Code : ASCE7(2016)

Description :

Wind Load Parameters

Basic Wind Speed : 85 mile/h

Exposure Category : C

Mean Roof Height : 50 m

Topographic Effects ...

Ground Elevation Factor: Kex 1 Key 1

Directional Factor : Kdx 0.85 Kdy 0.85

Rigid Structure Flexible Structure

Gust Effect Factor : Gx 0.85 Gy 0.85

Load Evaluation Using Force Coefficient

Force Coefficient : 1

Wind Eccentricity

X-Dir. (Wx) : Positive Negative None

Y-Dir. (Wy) : Positive Negative None

Wind Pressure > Velocity Pressure 설정

Add/Modify Velocity Pressure

Velocity Pressure Name :

Wind Load Code : ASCE7(2016)

Wind Load Parameters

Basic Wind Speed : 85 mile/h

Exposure Category : B

Mean Roof Height : 50 m

Include Topographic Effects

Topographic Factor at Building Ground Level

Kzt : 1

Auto Calculate

OK Cancel

- 공기 밀도를 고려한 계수(Ke factor, Ground elevation factor)를 곱하여 속도압을 산정합니다.
Ke factor는 ASCE 7-16 Table 26.9-1에 따라 결정됩니다.

[ASCE 7-16 26.10]

$$q_z = 0.00256K_zK_{zt}K_dK_eV^2 \text{ (lb/ft}^2\text{)}; V \text{ in mi/h} \quad (26.10-1)$$

WIND LOADS BASED ON ASCE7-16 [UNIT: kN, m]

Design Wind Loads	: F = p + A
Design Wind Pressure	: p = qz * G * Cp1 - qh * G * Cp2
Velocity Pressure at Design Height z [psf]	: qz = 0.00256 * Kd * Kz * Kzt * Ke * V^2
Velocity Pressure at Mean Roof Height [psf]	: qh = 0.00256 * Kd * Kh * Kht * Ke * V^2
Calculated Value of qh for X-dir [psf]	: qh = 0.65
Calculated Value of qh for Y-dir [psf]	: qh = 0.65
Basic Wind Speed [mph]	: V = 85.00
Directionality Factor for X-dir Wind Loads	: Kdx = 0.85
Directionality Factor for Y-dir Wind Loads	: Kdy = 0.85
Ground Elevation Factor	: Kex = 1.00
Ground Elevation Factor	: Key = 1.00
Exposure Category	: C
Mean Roof Height	: H = 50.00
Nominal height of the atmospheric boundary layer	: Zg = 274.32
3-s gust-speed power law exponent	: alpha = 9.50
Kz at Mean Roof Height	: Kh = 1.40

[midas Gen V945 R1] ASCE7-16, ASCE7-22 기준 Wind Load, Wind Pressure 지원 (계속)

- ASCE 7-22 기준 Kh, Kz 변경 및 풍압계산 시 Kd factor 반영

Load > Static Loads > Wind Loads, Wind Pressure

Wind Load 설정

Add/Modify Wind Load Specification

Load Case Name : WX

Wind Load Code : ASCE7(2022)

Description :

Wind Load Parameters

Basic Wind Speed : 85 mile/h

Exposure Category : C

Mean Roof Height : 50 m

Topographic Effects ...

Ground Elevation Factor: Kex 1 Key 1

Directional Factor : Kdx 0.85 Kdy 0.85

Rigid Structure Flexible Structure

Gust Effect Factor : Gx 0.85 Gy 0.85

Load Evaluation Using Force Coefficient

Force Coefficient : 1

Wind Eccentricity

X-Dir, (Wx) : Positive Negative None

Y-Dir, (Wy) : Positive Negative None

Wind Pressure > Velocity Pressure 설정

Add/Modify Velocity Pressure

Velocity Pressure Name :

Wind Load Code : ASCE7(2022)

Wind Load Parameters

Basic Wind Speed : 85 mile/h

Exposure Category : B

Mean Roof Height : 50 m

Include Topographic Effects

Topographic Factor at Building Ground Level

Kzt : 1

Auto Calculate

OK Cancel

- 풍향의 영향을 고려한 계수(Kd factor, Wind direction factor)를 적용하여 설계 풍압을 산정합니다.

- 높이에 따라 산정되는 Kh, Kz 값에 대한 기준 내용 변경 사항을 고려하여 속도압을 산정합니다.

[ASCE 7-22 27.3]

$$p = qK_dGC_p - q_iK_d(GC_{pi}) \quad (27.3-1)$$

[ASCE 7-22 26.10]

$$q_z = 0.00256K_zK_{zt}K_dK_eV^2 \text{ (lb/ft}^2\text{)}; V \text{ in mi/h} \quad (26.10-1)$$

WIND LOADS BASED ON ASCE7-22 [UNIT: kN, m]

Design Wind Loads	: F = p * A
Design Wind Pressure	: p = qz*Kd*G* Cp1 - qh*Kd*G* Cp2
Velocity Pressure at Design Height z [psf]	: qz = 0.00256*(Kz+Kzt+Ke)*V^2
Velocity Pressure at Mean Roof Height [psf]	: qh = 0.00256*(Kh+Kht+Ke)*V^2
Calculated Value of qh for X-dir [psf]	: qh = 0.77
Calculated Value of qh for Y-dir [psf]	: qh = 0.77
Basic Wind Speed [mph]	: V = 85.00
Directionality Factor for X-dir Wind Loads	: Kdx = 0.85
Directionality Factor for Y-dir Wind Loads	: Kdy = 0.85
Ground Elevation Factor	: Kex = 1.00
Ground Elevation Factor	: Key = 1.00
Exposure Category	: C
Mean Roof Height	: H = 50.00
Nominal height of the atmospheric boundary layer	: Zg = 749.81
3-s gust-speed power law exponent	: alpha = 9.80
Kz at Mean Roof Height	: Kh = 1.39

[midas Gen V945 R1] RC 부재 Checking Result Dialog Rebar Detail 출력

- RC Beam, Column, Brace, Wall Checking 시 Rebar Detail 출력
- KDS 41 20 : 2022, ACI318-19, ACI318-14, Eurocode2:04, NSR-10, NSCP 2015, NTC-DCEC(2017) 기준 검토 시 지원

Design > RC Design > Concrete Code Check > Beam Checking, Column Checking, Brace Checking, Wall Checking

▪ Checking Result Dialog 옵션 설정

KDS 41 20 : 2022 RC-Beam Checking Result Dialog

Code : KDS 41 20 : 2022 Unit : kN , m

Sorted by Member Strength
 Property Rebar Detail

▪ Rebar Detail NG 시 CHK 출력

RC Beam	Column	Wall
P : Rebar with Positive Moment N : Rebar with Negative Moment V : StIRRup T : Sidebar with Torsion	M : Main rebar ratio V : Hoop J : Hoop in Joint	V : Vertical rebar H : Horizontal rebar B : Hoop in Boundary area

RC Beam

MEMB	SECT	Section		fck	fy	POS	CHK	Main Rebar (Top)					Main Rebar (Bottom)					StIRRup			
		Bc	Hc					p.max (%)	p.use (%)	p.min (%)	s.max	s.use	p.max (%)	p.use (%)	p.min (%)	s.max	s.use	Av.use	Av.min	s.max	s.use
0		G1		24000.0		I	OK	2.464	1.239	0.080	0.2558	0.0867	3.051	0.603	0.276	0.2558	0.1300	0.0000	0.0000	0.3066	0.3000
211	<input type="checkbox"/>	0.400	0.700	400000		M	OK	2.448	0.603	0.184	0.2558	0.1300	2.448	0.603	0.276	0.2558	0.1300	0.0000	0.0000	0.3150	0.3100
10.200		0.000	0.000	400000		J	OK	2.464	1.239	0.292	0.2558	0.0867	3.051	0.603	0.276	0.2558	0.1300	0.0000	0.0000	0.3066	0.3000

RC Column

MEMB	SECT	Section		fck	fy	CHK	Main Rebar (%)			Hoop						
		Bc	Hc				Height	fys	p.max	p.use	p.min	POS	Avy.use	Avy.min	Avz.use	Avz.min
0		C1		24000.0	400000					End	0.0001	-	0.0001	0.0001	0.3552	0.2000
105	<input type="checkbox"/>	0.800	0.800	4.0000	400000	OK	3.000	1.089	1.000	Mid	0.0001	-	0.0001	0.0001	0.3552	0.2000

RC Wall

WID	Story	Wall Mark		fck	fy	CHK	V-Rebar					H-Rebar				
		Lw	HTw				hw	fys	p.max(%)	p.use(%)	p.min(%)	s.max	s.use	p.use(%)	p.min(%)	s.max
1		W1		24000.0	400000											
5F	<input type="checkbox"/>	7.2000	4.0000	0.2000	400000	OK	4.000	0.352	0.120	0.4500	0.4000	0.255	0.200	0.4500	0.2800	

midas DESIGN⁺

RC			STEEL		SRC	ALU
 Slab	 Beam	 Column	 Beam+Column	 Base Plate	 Comp. Beam	 Beam+Column
 Shear Wall	 Footng	 Basement Wall	 Bolt Connection	 Crane Girder	 Column	 Beam+Column
 Buttress	 Stair	 Corbel+Bracket	 Purlin+Girth	 Web Opening	 CFT Column	
 Anchor Bolt	 Beam Table	 Batch Wall	 Stair	 Welding		

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Design+ V.495

[midas Design+ V495 R1] ACI318M-19, ACI318-19 설계 지원

- ACI 기준 강도제한, 변형률, 전단강도 산정 등 개정내용 반영

Option > Design Code > RC

10. Check shear capacity

(1) Calculate maximum space

- $\phi = 0.750$
- $s_{max} = \min(16D_{MainBar}, 48D_{Stirrup}, B, D) = 457\text{mm}$

(2) Calculate Shear Strength (Direction X)

- $s = 100\text{mm} < s_{max} = 457\text{mm} \rightarrow \text{O.K.}$
- $\rho_w = 0.0147$
- $V_{c,max} = 0.42 \sqrt{f'_c} b_w d = 450\text{kN}$
- $V_{c1} = [0.17 \sqrt{f'_c} + N_u / (6 A_g)] b_w d = 328\text{kN}$
- $V_{c2} = [0.66 (\rho_w)^{1/3} \sqrt{f'_c} + N_u / (6 A_g)] b_w d = 319\text{kN}$
- $V_c = \min[V_{c1}, V_{c2}, V_{c,max}] = 319\text{kN} (A_v \geq A_{v,min})$
- $\phi V_c = 246\text{kN}$
- $\phi V_s = \phi \frac{A_v f_{yt} d}{s} = 280\text{kN}$
- $\phi V_n = \phi V_c + \phi V_s = 526\text{kN}$
- $\phi V_{nmax} = 777\text{kN}$
- $V_u / \phi V_{nmax} = 0.286 \rightarrow \text{O.K.}$
- $V_u / \phi V_n = 0.422 \rightarrow \text{O.K.}$

(3) Calculate Shear Strength (Direction Y)

- $s = 100\text{mm} < s_{max} = 457\text{mm} \rightarrow \text{O.K.}$
- $\rho_w = 0.0177$
- $V_{c,max} = 0.42 \sqrt{f'_c} b_w d = 450\text{kN}$
- $V_{c1} = [0.17 \sqrt{f'_c} + N_u / (6 A_g)] b_w d = 328\text{kN}$
- $V_{c2} = [0.66 (\rho_w)^{1/3} \sqrt{f'_c} + N_u / (6 A_g)] b_w d = 330\text{kN}$
- $V_c = \min[V_{c1}, V_{c2}, V_{c,max}] = 328\text{kN} (A_v \geq A_{v,min})$
- $\phi V_c = 240\text{kN}$

[midas Design+ V495 R1] ACI318M-19, ACI318-19 설계 지원 (계속)

- 철근 정착, 이음길이 계산 반영

Tool > Dev. Splice Calculator

Development & Splicing

Calculation Type
Development Length, Tension (Simple)

Formula
$$l_d = \frac{f_y \psi_t \psi_e \psi_s \psi_g}{k \lambda \sqrt{f_c}} d_b$$

(k : 2.1, 1.7, 1.4, 1.1)

Material Strength
fy: 400 MPa
fck: 24 MPa

Rebar
Size: #8
Cc: 40.00 mm
s: 40.00 mm

Applicable Factor
ψt: 1.000
ψe: 1.000
λ: 1.000

Use As Ratio
As,req: 0.00 mm²
As,app: 0.00 mm²

Calculation Result: 1885.36 mm

Development & Splicing

Calculation Type
Development Length, Tension (Detail)

Formula
$$l_d = \frac{f_y}{1.1 \lambda \sqrt{f_c}} \left(\frac{C_b + K_{tr}}{d_b} \right) d_b$$

Material Strength
fy: 400 MPa
fck: 24 MPa

Rebar
Size: #8
Cb: 40.00 mm

Applicable Factor
ψt: 1.000
ψe: 1.000
λ: 1.000
Ktr: 0.00

Calc Ktr
Stirrup: #3
Space: 100.00 mm
ns: 2
nl: 4
fyt: 400 MPa

Use As Ratio
As,req: 0.00 mm²
As,app: 0.00 mm²

Calculation Result: 1197.21 mm

Development & Splicing

Calculation Type
Development Length, Compression

Formula
$$l_d = \max \left(\frac{0.24 f_y \psi_r}{\lambda \sqrt{f_c}}, 0.043 f_y \psi_r \right) d_b$$

Material Strength
fy: 400 MPa
fck: 24 MPa

Rebar
Size: #8

Applicable Factor
ψr: 1.000
λ: 1.000

Use As Ratio
As,req: 0.00 mm²
As,app: 0.00 mm²

Calculation Result: 497.74 mm

Development & Splicing

Calculation Type
Development Length, Tension (Hook)

Formula
$$l_d = \left(\frac{f_y \psi_e \psi_r \psi_s \psi_c}{23 \lambda \sqrt{f_c}} \right) d_b^{1.5}$$

Material Strength
fy: 400 MPa
fck: 24 MPa

Rebar
Size: #8

Applicable Factor
ψe: 1.000
λ: 1.000
ψr: 1.000
ψs: 1.000

Use As Ratio
As,req: 0.00 mm²
As,app: 0.00 mm²

Calculation Result: 376.54 mm

Development & Splicing

Calculation Type
Splice Length, Tension

Formula
Class A: $l_s = 1.0 l_d$
Class B: $l_s = 1.3 l_d$

Material Strength
fy: 400 MPa
fck: 24 MPa

Rebar
Size: #8
Cb: 40.00 mm

Splice Grade
 Class A Class B

Applicable Factor
ψt: 1.000
ψe: 1.000
λ: 1.000
Ktr: 0.00

Calc Ktr
Stirrup: #3
Space: 100.00 mm
ns: 2
nl: 4
fyt: 400 MPa

Use As Ratio
As,req: 0.00 mm²
As,app: 0.00 mm²

Calculation Result: 1197.21 mm

Development & Splicing

Calculation Type
Splice Length, Compression

Formula
$$f_y \leq 420 : l_s = 0.071 f_y d_b$$

$$f_y > 420 : l_s = (0.13 f_y - 24) d_b$$

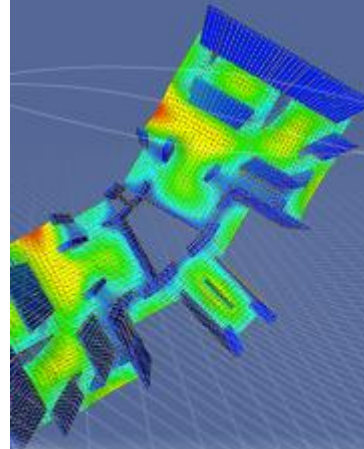
Material Strength
fy: 400 MPa
fck: 24 MPa

Rebar
Size: #8

Splice Length
ls, tension: 0.00 mm

Calculation Result: 721.36 mm

midas SDS *Slab & basement Design System*



midas SDS is a specialized Slab and Foundation (Pile/Mat) Analysis and optimal design system.

It maximizes efficiency and productivity in complex slab modeling by offering user-friendly graphic interface and 100% data conversion with midas Gen and ADS.

Unlike grid-based mesh generation, Object oriented modeling enables to freely model the structures of complex floor layouts without any restriction of mesh lines.

As a result, more accurate analysis results can be obtained through automatically generated elements.

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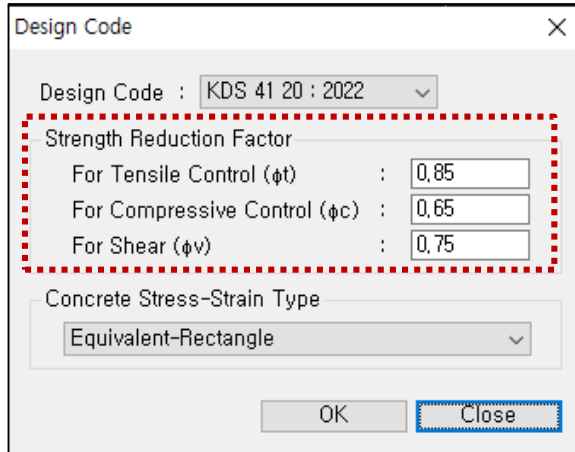
SDS V.410

[midas SDS V410 R1] 콘크리트 휨강도감소계수 계산 개선

- KCI-USD07, KCI-USD12, KDS 41 30 : 2018, KDS 41 20 : 2022 기준 검토 시 지원
- Flexural Design, Flexural Checking 시 고려

Design > Design Code

Design > Flexural Design Result, Flexural Checking Result



* 휨강도감소계수는 인장지배단면 강도감소계수와 압축지배단면 강도감소계수를 최대, 최소값으로 하여 변형률에 따라 보간하여 계산합니다.

(KDS 14 20 10 : 2021 4.2.3 (2) 참고, Gen과 동일한 방식)

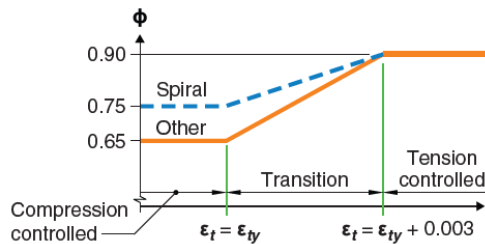


Fig. R21.2.2b—Variation of φ with net tensile strain in extreme tension reinforcement, εt.

■ ACI318-19 이미지 참고

```

=====
*. midas SDS (KDS 41 20 : 2022) - Flexural Checking Maximum Result   Ver.410
=====
<< BOTTOM >>
-----
-. Information of Parameters.
Elem No. : 230
Node No. : 39
LCB No. : gLCB1
Materials : fck = 24.0000 N/mm^2
           Fy = 400.0000 N/mm^2
Thickness : 500.0000 mm
Covering : dB = 30.0000 mm
           dT = 30.0000 mm

-. Information of Design.
b = 1.0 mm (by Unit Length).
d = 470.0000 mm

-. Information of Strength Reduction Factor.
[ KDS 14 20 10 : 2021, 4.2.3 ]
phi_c = 0.650
phi_t = 0.850
et = 0.00457
ety = 0.00200
etu = 0.00500
ety < et < etu : Transition
phi = phi_c + (phi_t-phi_c)/(etu - ety)*(et - ety) = 0.828

-. Information of Required reinforcement
Mu_cur_max = 28797.4281 N+mm/mm (gLCB1)
Mu_opp_max = 143750.8660 N+mm/mm (gLCB1)
Fact_dist = | Mu_cur_max / (Mu_cur_max + Mu_opp_max) | = 1.0000
rho_min1 = 0.0020 (by temperature reinforcement)
rho_min2 = 0.0036 (by minimum flexural reinforcement in slab)
rho_cal = 0.0004 (by design moment)
rho_req = max[ rho_min1*Fact_dist, rho_min2, rho_cal ] = 0.0021

-. Information of Moments and Result.
Rein. Bar : 032 @100
As_req = 1.0000 mm^2/mm( 1000.0000 mm^2/m)
As_use = 7.9420 mm^2/mm( 7942.0000 mm^2/m)

Stress-strain relation : Equivalent-rectangle
[ KDS 14 20 20 : 2021, Table 4.1-2 ]
eta = 1.000
Mn = As_use*Fy*{ d - (As_use*Fy / (2*eta+0.85*fck*b)) } = 1245741.6314 N+mm/mm

phiMn = phi * Mn = 1031296.9293 N+mm/mm
Mu = 28797.4281 N+mm/mm
RatM = Mu / phiMn = 0.028 < 1.0 ----> O.K !
    
```

이외 주요 개선사항 및 버그수정사항은 아래와 같습니다. 고객 여러분의 관심과 프로그램 개선 참여에 깊은 감사 드립니다.

[midas Gen 2024 V945 R1]

- [Seismic Performance] RC 보, 기둥, 벽체 강도 산정 시 $V_{s,max}$ 를 고려할 수 있도록 수정
- [Seismic Performance] Pushover Hinge Unloading Stiffness Type 수정
- [Seismic Performance] [MOE, KISTEC]내진성능평가 조적 스트럿 힌지 속성 자동계산 시 적용되는 기둥 전단강도 계산 수정
- [Seismic Performance] [AIK-G-001-2021] Initialize Hinge Properties 하지 않고 Elastic Analysis로 힌지속성 자동계산할 수 있도록 개선
- [Seismic Performance] [AIK-G-001-2021] Time-history +max, -min 의 Average 출력 추가
- [Seismic Performance] [AIK-G-001-2021] Time-history All 출력 삭제
- RC Beam 휨철근 최대간격 산정 시 사용하중 상태의 f_s 값 적용할 수 있도록 Concrete Design Code 옵션 추가 (KDS 14 20 20 : 2022 4.2.3 (4))
- KDS 41 20 : 2022 기준 RC 부재 검토 시 메모리 성능 개선
- KDS 41 20 : 2022 기준 RC 보 Checking 시 사용자 입력 철근 강도 적용 수정
- KBC(2009)~KDS(41-12:2022) 기준 풍하중 Gust Factor 자동계산 시 Topographic Effect Height from Ground Level을 고려하지 않도록 수정
- Member Assignment, Load Cases for Wind Direction, Seismic Component Type 오류로 파일 오픈 오류가 발생하는 경우, 해당 데이터를 삭제하고 파일을 오픈할 수 있도록 개선
- Torsional Irregularity Check, Torsional Amplification Factor Result Table X, Y 방향별로 구분하여 출력하도록 개선
- [Design+ 연동] Batch Wall 특수전단벽 정보 연동 수정

[midas Design+ V495 R1]

- 대용량 파일 저장 시 진행 상황 표시할 수 있도록 개선
- Batch Wall Apply, Design, Check 단축키 적용할 수 있도록 개선
- Batch Wall 단부 보강근 편집열을 숨길 수 있도록 개선
- Combined Footing 검토 시 모멘트를 고려할 수 있도록 개선
- 지하외벽 하부층 전단력이 0으로 표기되는 경우 수정
- RC 보 장기처짐 검토 시 단면 2차모멘트 적용값 출력 수정

[midas ADS V285 R1]

- KBC(2009)~KDS(41 12 : 2022) 기준 풍하중 Gust Factor 자동계산 시 Topographic Effect Height from Ground Level을 고려하지 않도록 수정
- KDS(41 12 : 2022) 기준 풍하중 적용 시 Wind Response만 체크한 경우에도 Vibration Mode 입력할 수 있도록 수정